SIMPSON
Strong-Tie

AT-HP™ är en mångsidig ankarmassa med hög styrka. Nästan luktfri och därför även lämplig att användas inomhus. AT-HP™ är helt fri från styren vilket gör användandet säkrare. För infästning i betong, murverk och lättbetong. Montage sker med en standard 300 ml patronspruta.

CE

ETA

[ETA-19/0265](ETA-19/0265)

EGENSKABER

Material

- Ankarmassa av metakryl-typ
- Styrenfri och luktfri

Fördelar

- Montage i fuktiga hål är godkänt
- Färgförändring visar korrekt blandning
- Spänningsfri fastsättning
- Förseglar det borrade hålet fullständigt
- Nästan luktfri

ANVäNDNING

Monteras på

- Betong C20-C50 (Option 1)
- Sprucken betong C20-C50 (Option 1)
- Lättbetong
- Ihåligt- och massivt tegel

Användningsområden

- Gängstänger / pinnbultar M8 - M24
- Armeringsjärn Ø8 - Ø25 mm
- Stålkonstruktioner
- Skensystem
- Fasader
- Balkar
- Markiser
- Maskiner
- Balkonger
- Hyllor
- mm.

Hedgaardsvej 11 8300 Odder
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TEKNISK DATA

Références

Art. nr.	Product information						
	DB nr.	NOBB nr.	Grey color	Beige color	Content [ml]	Weight [kg]	Packaging qty [pcs]
ATHP300G-FR	-	-	x	-	300	0.575	12
ATHP420G-FR	-	-	x	-	420	0.828	12
ATHP420BG-PL	-	-	x	-	420	0.828	12
ATHP300BG-PL	-	-	x	-	300	0.575	12
ATHP300BG-DK	2099761	56432785	-	-	300	0.575	12

Design resistance – Tension – NRd [kN] – hef = 8d – Carbon steel 5.8

Art. nr.	Design resistance – h_{ef} = 8d – Carbon steel 5.8							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	10.7	12	12	12
AT-HP + LMAS M10	-	-	-	-	15.9	17.8	19.3	19.3
AT-HP + LMAS M12	8.4	8.8	9	9.2	21.7	24.3	26.7	28
AT-HP + LMAS M16	15	15.6	16.1	16.4	34.3	38.4	42.2	44.6
AT-HP + LMAS M20	-	-	-	-	50.2	56.3	61.8	65.3
AT-HP + LMAS M24	-	-	-	-	67.5	75.6	83.1	87.8

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is $\sigma_L + \sigma_R \le 0$. In the absence of detailed verification $\sigma_R = 3$ N/mm² can be assumed (σ_L equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Tension – NRd [kN] – hef = 12d – Carbon steel 5.8

Art. nr.	Design resistance – h_{ef} = 12d – Carbon steel 5.8							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	12	12	12	12
AT-HP + LMAS M10	-	-	-	-	19.3	19.3	19.3	19.3
AT-HP + LMAS M12	12.7	13.2	13.5	13.8	28	28	28	28
AT-HP + LMAS M16	22.5	23.4	24.1	24.5	51.4	52.7	52.7	52.7

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Art. nr.	Design resistance – h_{ef} = 12d – Carbon steel 5.8							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M20	-	-	-	-	75.4	82	82	82
AT-HP + LMAS M24	-	-	-	-	101.3	113.4	118	118

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
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3. Concrete is considered non-cracked when the tensile stress within the concrete is $\sigma_L + \sigma_R \le 0$. In the absence of detailed verification σ_R = 3 N/mm² can be assumed (σ_L equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Tension – NRd [kN] – hef = 8d – Stainless steel A4-70

Art. nr.	Design resistance – h_{ef} = 8d – Stainless steel A4-70							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
ATHP300BG-PL	-	-	-	-	-	-	-	-
AT-HP + LMAS M8	-	-	-	-	10.7	12	13.2	13.9
AT-HP + LMAS M10	-	-	-	-	15.9	17.8	19.6	20.7
AT-HP + LMAS M12	8.4	8.8	9	9.2	21.7	24.3	26.7	28.2
AT-HP + LMAS M16	15	15.6	16.1	16.4	34.3	38.4	42.2	44.6
AT-HP + LMAS M20	-	-	-	-	50.2	56.3	61.8	65.3
AT-HP + LMAS M24	-	-	-	-	67.5	75.6	83.1	87.8

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
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Design resistance – Tension – NRd [kN] – hef = 12d – Stainless steel A4-70

Art. nr.	Design resistance – h_{ef} = 12d – Stainless steel A4-70							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	13.9	13.9	13.9	13.9
AT-HP + LMAS M10	-	-	-	-	21.9	21.9	21.9	21.9

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Art. nr.	Design resistance – h_{ef} = 12d – Stainless steel A4-70							
	Tension - N_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M12	12.7	13.2	13.5	13.8	31.6	31.6	31.6	31.6
AT-HP + LMAS M16	22.5	23.4	24.1	24.5	51.4	57.6	58.8	58.8
AT-HP + LMAS M20	-	-	-	-	75.4	84.4	92	92
AT-HP + LMAS M24	-	-	-	-	101.3	113.4	124.6	131.7

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is $\sigma L +\sigma R ≤ 0$. In the absence of detailed verification $\sigma R = 3$ N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Shear – VRd [kN] – hef = 8d – Carbon steel 5.8

Art. nr.	Design resistance – h_{ef} = 8d – Carbon steel 5.8							
	Shear - V_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	7.2	7.2	7.2	7.2
AT-HP + LMAS M10	-	-	-	-	12	12	12	12
AT-HP + LMAS M12	16.8	16.8	16.8	16.8	16.8	16.8	16.8	16.8
AT-HP + LMAS M16	30	31.2	31.2	31.2	31.2	31.2	31.2	31.2
AT-HP + LMAS M20	-	-	-	-	48.8	48.8	48.8	48.8
AT-HP + LMAS M24	-	-	-	-	70.4	70.4	70.4	70.4

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is $\sigma L +\sigma R ≤ 0$. In the absence of detailed verification $\sigma R = 3$ N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Shear – VRd [kN] – hef = 12d – Carbon steel 5.8

Art. nr.	Design resistance – h_{ef} = 12d – Carbon steel 5.8							
	Shear - V_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	7.2	7.2	7.2	7.2

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Art. nr.	Design resistance – h_{ef} = 12d – Carbon steel 5.8							
	Shear - V_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M10	-	-	-	-	12	12	12	12
AT-HP + LMAS M12	16.8	16.8	16.8	16.8	16.8	16.8	16.8	16.8
AT-HP + LMAS M16	31.2	31.2	31.2	31.2	31.2	31.2	31.2	31.2
AT-HP + LMAS M20	-	-	-	-	48.8	48.8	48.8	48.8
AT-HP + LMAS M24	-	-	-	-	70.4	70.4	70.4	70.4

Concrete :
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3. Concrete is considered non-cracked when the tensile stress within the concrete is σL +σR ≤ 0. In the absence of detailed verification σR = 3 N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Shear – VRd [kN] – hef = 8d – Stainless steel A4-70

Art. nr.	Design resistance – h_{ef} = 8d – Stainless steel A4-70							
	Shear - V_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	8.3	8.3	8.3	8.3
AT-HP + LMAS M10	-	-	-	-	12.8	12.8	12.8	12.8
AT-HP + LMAS M12	16.9	17.6	18.1	18.4	19.2	19.2	19.2	19.2
AT-HP + LMAS M16	30	31.2	32.1	32.7	35.3	35.3	35.3	35.3
AT-HP + LMAS M20	-	-	-	-	55.1	55.1	55.1	55.1
AT-HP + LMAS M24	-	-	-	-	79.5	79.5	79.5	79.5

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is σL +σR ≤ 0. In the absence of detailed verification σR = 3 N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Shear - VRd [kN] – hef = 12d – Stainless steel A4-70

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Art. nr.	Design resistance – h_{ef} = 12d – Stainless steel A4-70							
	Shear - V_{Rd} [kN]							
	Cracked concrete				Non-cracked concrete			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + LMAS M8	-	-	-	-	8.3	8.3	8.3	8.3
AT-HP + LMAS M10	-	-	-	-	12.8	12.8	12.8	12.8
AT-HP + LMAS M12	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2
AT-HP + LMAS M16	35.3	35.3	35.3	35.3	35.3	35.3	35.3	35.3
AT-HP + LMAS M20	-	-	-	-	55.1	55.1	55.1	55.1
AT-HP + LMAS M24	-	-	-	-	79.5	79.5	79.5	79.5

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is σL +σR ≤ 0. In the absence of detailed verification σR = 3 N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Bending moment – MRd [Nm] – Concrete

Art. nr.	Design resistance – Bending moment – M_{Rd} [Nm]	
	Carbon steel 5.8	Stainless steel A4-70
AT-HP + LMAS M8	15.2	16.7
AT-HP + LMAS M10	29.6	34
AT-HP + LMAS M12	52.8	59
AT-HP + LMAS M16	133.6	149.4
AT-HP + LMAS M20	260.8	291
AT-HP + LMAS M24	448.8	502.6

Concrete :
1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing s ≥ 15 cm (any diameter) or with a rebar spacing s ≥ 10 cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges (c ≤ max [10 hef; 60d]) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is σL +σR ≤ 0. In the absence of detailed verification σR = 3 N/mm² can be assumed (σL equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Tension – NRd [kN] – Rebar

Art. nr.	Design resistance – N_{Rd} – Carbon steel 5.8 [kN]							
	Non-cracked concrete							
	h_{ef} = 8d				h_{ef} = 12d			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
ATHP300G-FR	-	-	-	-	-	-	-	-
ATHP420G-FR	-	-	-	8.1	-	-	-	-
ATHP420BG-PL	-	-	-	-	-	-	-	-

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Art. nr.	Design resistance – N_{Rd} – Carbon steel 5.8 [kN]							
	Non-cracked concrete							
	h_{ef} = 8d				h_{ef} = 12d			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
ATHP300BG-PL	-	-	-	-	-	-	-	-
AT-HP + LMAS M8	-	-	-	13.6	-	-	-	-
AT-HP + LMAS M10	-	-	-	18.3	-	-	-	-
AT-HP + LMAS M12	-	-	-	24.9	-	-	-	-
AT-HP + LMAS M16	-	-	-	34.8	-	-	-	-
AT-HP + LMAS M20	-	-	-	47.2	-	-	-	-
AT-HP + LMAS M24	-	-	-	68	-	-	-	-
AT-HP + Ø8	6.3	7	7.7	8.1	9.4	10.5	11.5	12.2
AT-HP + Ø10	10.5	11.7	12.9	13.6	15.7	17.6	19.3	20.4
AT-HP + Ø12	14.1	15.8	17.3	18.3	21.1	23.6	26	27.4
AT-HP + Ø14	19.1	21.4	23.6	24.9	28.7	32.2	35.3	37.3
AT-HP + Ø16	23.2	26	28.6	34.8	34.8	39	42.8	52.2
AT-HP + Ø20	36.3	40.6	44.6	47.2	54.4	61	66.9	70.8
AT-HP + Ø25	52.3	58.6	64.4	68	78.5	87.9	96.6	102.1

Design resistance – Shear – VRd [kN] – Rebar

Art. nr.	Design resistance – V_{Rd} – Carbon steel 5.8 [kN]							
	Non-cracked concrete							
	h_{ef} = 8d				h_{ef} = 12d			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
AT-HP + Ø8	9.3	9.3	9.3	9.3	9.3	9.3	9.3	9.3
AT-HP + Ø10	14.7	14.7	14.7	14.7	14.7	14.7	14.7	14.7
AT-HP + Ø12	20.7	20.7	20.7	20.7	20.7	20.7	20.7	20.7
AT-HP + Ø14	28	28	28	28	28	28	28	28
AT-HP + Ø16	36.7	36.7	36.7	36.7	36.7	36.7	36.7	36.7
AT-HP + Ø20	57.3	57.3	57.3	57.3	57.3	57.3	57.3	57.3
AT-HP + Ø25	90	90	90	90	90	90	90	90
ATHP300BG-DK	-	-	-	-	-	-	-	-

Design resistance – Bending moment – MRd [Nm] – Rebar

Art. nr.	Design resistance – Bending moment – M_{Rd} [Nm]
AT-HP + Ø8	22
AT-HP + Ø10	43.3
AT-HP + Ø12	74.7
AT-HP + Ø14	118.7
AT-HP + Ø16	176.7
AT-HP + Ø20	345.3
AT-HP + Ø25	674.7

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MONTERING

Exponeringstid

Temperatur	-5°C	0°C	5°C	10°C	20°C	30°C
Arbetstid	45min	15min	12min	9min	4min	1min
Laddningstid	9h	4h	1h30	60min	30min	20min

Drilling methods

Solid brick/concrete	Percussion/hammer drilling
Hollow/perforated brick	Rotation drilling
Aerated concrete	Percussion/hammer drilling

Borra hål.
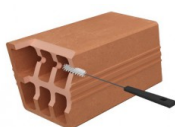
Rengör hålet med stålborste och blåspump (blås x5, borsta x4 och blås x5).
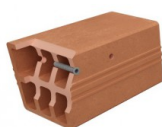
Indsæt en sihylse

Tryck ut massa tills strängen har enhetlig färg.

Stick in pinnbulten med en roterande rörelse och låt massan härda.

Fäst arbetsstycket och dra åt med rekommenderat moment med en kalibrerad momentnyckel.

Borra hål.

Rengör hålet med stålborste och blåspump (blås x5, borsta x4 och blås x5).

Fyld hullet 1/2 - 2/3 op med klæbemørtel ved hjælp af pump fra bunden og udad

Stick in pinnbulten med en roterande rörelse och låt massan härda.

Fäst arbetsstycket och dra åt med rekommenderat moment med en kalibrerad momentnyckel.

Installation parameters – Concrete

Art. nr.	Installation parameters - Concrete					
	Ø drilling [d_0] [mm]	Max. fixture hole Ø [d_f] [mm]	Drilling depth (8d) [$h_0=h_{ef}=8d$] [mm]	Drilling depth (12d) [$h_0=h_{ef}=12d$] [mm]	Wrench size [SW]	Installation torque [T_{inst}] [Nm]
AT-HP + LMAS M8	10	9	64	96	13	10
AT-HP + LMAS M10	12	12	80	120	17	20
AT-HP + LMAS M12	14	14	96	144	19	30
AT-HP + LMAS M16	18	18	128	192	24	60
AT-HP + LMAS M20	24	22	160	240	30	90
AT-HP + LMAS M24	28	26	192	288	36	140

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Rättigheterna till upplysningar i detta dokument tillhör Simpson Strong-Tie©
Gäller endast i förbindelse med produkter som sålts av Simpson Strong-Tie©

AT-HP - Ankarmassa

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Spacing, edge distances and member thickness - Concrete

Art. nr.	Spacing, edge distance and member thickness - Concrete									
	Effective embedment depth (8d) $[h_{ef,8d}]$ [mm]	Characteristic spacing for $h_{ef,8d}$ $[S_{cr,N}]$ [mm]	Characteristic edge distance for $h_{ef,8d}$ $[c_{cr,N}]$ [mm]	Min. member thickness for $h_{ef,8d}$ $[h_{min}]$ [mm]	Effective embedment depth (12d) $[h_{ef,12d}]$ [mm]	Characteristic spacing for $h_{ef,12d}$ $[S_{cr,N}]$ [mm]	Characteristic edge distance for $h_{ef,12d}$ $[c_{cr,N}]$ [mm]	Min. member thickness for $h_{ef,12d}$ $[h_{min}]$ [mm]	Min. spacing $[S_{min}]$ [mm]	Min. edge distance $[C_{min}]$ [mm]
AT-HP + LMAS M8	64	192	96	100	96	288	144	100	40	40
AT-HP + LMAS M10	80	240	120	110	120	360	180	150	50	50
AT-HP + LMAS M12	96	288	144	126	144	432	216	174	60	60
AT-HP + LMAS M16	128	384	192	158	192	576	288	222	80	80
AT-HP + LMAS M20	160	480	240	190	240	720	360	270	100	100
AT-HP + LMAS M24	192	576	288	222	288	864	432	318	120	120

Installation parameters – Rebar

Art. nr.	Installation parameters - Rebar		
	Ø drilling $[d_0]$ [mm]	Drilling depth (8d) $[h_0=h_{ef}=8d]$ [mm]	Drilling depth (12d) $[h_0=h_{ef}=12d]$ [mm]
AT-HP + Ø8	12	64	96
AT-HP + Ø10	14	80	120
AT-HP + Ø12	16	96	144
AT-HP + Ø14	18	112	168
AT-HP + Ø16	20	128	192
AT-HP + Ø20	25	160	240
AT-HP + Ø25	32	200	300

Spacing, edge distances and member thickness – Rebar

Art. nr.	Spacing, edge distance and member thickness - Rebar									
	Effective embedment depth (8d) $[h_{ef,8d}]$ [mm]	Characteristic spacing for $h_{ef,8d}$ $[S_{cr,N}]$ [mm]	Characteristic edge distance for $h_{ef,8d}$ $[c_{cr,N}]$ [mm]	Min. member thickness for $h_{ef,8d}$ $[h_{min}]$ [mm]	Effective embedment depth (12d) $[h_{ef,12d}]$ [mm]	Characteristic spacing for $h_{ef,12d}$ $[S_{cr,N}]$ [mm]	Characteristic edge distance for $h_{ef,12d}$ $[c_{cr,N}]$ [mm]	Min. member thickness for $h_{ef,12d}$ $[h_{min}]$ [mm]	Min. spacing $[S_{min}]$ [mm]	Min. edge distance $[C_{min}]$ [mm]
AT-HP + Ø8	64	192	96	100	96	288	144	100	40	40
AT-HP + Ø10	80	240	120	110	120	360	180	150	50	50
AT-HP + Ø12	96	288	144	126	144	432	216	174	60	60
AT-HP + Ø14	112	336	168	148	168	504	252	204	70	70
AT-HP + Ø16	128	384	192	168	192	576	288	232	80	80
AT-HP + Ø20	160	480	240	210	240	720	360	290	100	100
AT-HP + Ø25	200	600	300	264	300	900	450	364	125	125

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