



HALFEN HBT REBEND CONNECTION Technical Product Information



- DIBt Approval Z-21.8-2035
- Type tested acc. to approval and EUROCODE 2



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CAD drawings for HBT-Elements can be found in our CAD library under reinforcement technology. All drawing can be downloaded free at www.halfen.com. Following file formats are available: • CAD - DWG and DXF

A free DVD is also available if preferred. Our contact details, addresses, telephone and fax numbers can be found on the back cover of this catalogue Following approval and type test apply in this document:

 German National Technical Approval (DIBt Deutsches Institut f
ür Bautechnik) No. Z-21.8-2035 for HALFEN HBT Rebend Connection





Introduction and System Description

HBT - the connection solution

The HALFEN HBT is the first rebend connection with general building approval. The simplified calculation method according to the approval requires verification of only two basic load cases, this results in higher shear load capacity than previously.



Safety

- improved planning-reliability through general building authority approval based on real-world load capacity tests
- type tested load capacity tables
- more safety in planning and execution due to factory production and third party control

Simple

- simplified calculation concept with only two basic load cases
 shear load longitudinal to joint
 - shear load transverse to joint
- both shear and longitudinal load transfer using standard type profiles
- if required a verification of combined shear and longitudinal load in the concrete joint is possible
- standard type is suitable for constructive connections as well as for static critical connections – no risk of mix ups on-site or in the precast plant
- reduced case height ideal for thin elements or precast concrete elements with minimal concrete cover

HALFEN HBT Rebend connections allow a simple and effective connection of reinforced concrete elements, which are cast in different phases. All types of slabs, from floor slabs, walls and stairs can be subsequently cast with sufficient bond.

The rebar are bent and secured in a case with a back and a removable cover. Our profile cases are available in different widths. The case is cast into the concrete element; after striking the formwork, the case cover is removed and the rebar is straightened.

The customer can select either a single-row or double-row profile. The single-row profile has a regular spacing of holes in a single row penetrated by reinforcement bars designed for the required application; the double-row profile is similar but has two rows.

- rebar (8, 10, 12 mm)
 B500B steel (stainless steel
 B500 NR on request)
- the back is galvanised sheet metal with a specially profiled surface
- dimensionally stable, galvanised, sheet metal cover with a pre-punched hole to facilitate removal



Versatile

Optimal connections for a multitude of applications with 57 possible combinations of rebar and profile widths. A standardized range of product with element lengths of 0.8 m and 1.25 m are available for the most common applications.

Lots of reasons, one conclusion: safety, quality and protection – for you and your company.



Type Overview/Application Examples



Type Overview/Application Examples



Single-Row Profiles

Profile dimensions





Case dimensions [mm]										
profile	rebar Ø [mm]	width B	height H ₁	height H ₂						
HBT 55	8	58		24						
CC IGH	10	58		30						
HBT 85	10	86	12	30						
C0 101	12	00	12	36						
HBT 120	10	122		30						
TIDT 120	12	122		36						

Rebar layout			
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]
	10	12	7.5
standard element	15	8	10.0
L = 1250 mm	20	6	12.5
	25	5	12.5
	10	8	5.0
short element	15	6	2.5
L = 800 mm	20	4	10.0
	25	4	2.5

Other element lengths on request

Rebar types



See table on page 7 for rebar dimensions

Single-Row Profiles

Rebar dimension	Rebar dimensions [mm]											
profile	rebar Ø	rebar spacing		standard type 1			only for type 4					
				L = 1250 mm			L = 1250 mm	L ≥ 800 mm				
	[mm]	[cm]	h	l _ü y		h _{min}	l _{ü,max}	l _{ü,max}	b [mm]			
HBT 55		10		210			210	210				
	0	15	170		75	120	510	430				
	8	20	170	320	75		600	455				
		25					800	480	200500			
		10		200		120	200	200	200900			
	10	15	170		95		390	390				
	10	20	170	390			510	450				
		25						475				
HBT 85		10	170	390	95	120	430	400				
	10	15					510	425				
	10	20				120	600	450				
	2	25					000	475	250500			
		10		430			430	395				
	12	15	170		110	120	510	420				
	12	20	170	460	110	120	600	445				
		25					000	470				
HBT 120		10						400				
le de la companya de	10	15	170	390	95	120	600	425				
	10	20				.20		450				
~		25						475	on request			
		10						395				
	12	15	170	460	110	120	600	420				
	12	20						445				
		25						470				

Double-Row Profiles

Profile dimensions



Case dimer	nsions [mm]				
profile	rebar Ø [mm]	width B	height H ₁	height H ₂	b
HBT 80	8	86		24	58
	10	00		30	60
	8			24	88
HBT 120	10	122		30	90
	12			36	92
	8			24	116
HBT 150	10	150	12	30	118
	12		12	36	120
	8			24	152
HBT 190	10	186		30	154
	12			36	156
	8			24	188
HBT 220	10	222		30	190
	12			36	192



Rebar layout			
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]
	10	12	7.5
 standard element	15	8	10.0
L = 1250 mm	20	6	12.5
	25	5	12.5
	10	8	5.0
short element	15	6	2.5
L = 800 mm	20	4	10.0
	25	4	2.5

Other element lengths on request

Rebar types



Double-Row Profiles

profile	rebarØ	rebar	standard	type 5	type	es 5, 11, 22, 23	④, 33	types 6, 7, 8, 9																			
		spacing	L=1250 mm	71.7 -	0,00	L=1250 mm		type 6	type 7 ③, 8	type 9	L≥ 600 mm																
	[mm]	[cm]	h	lü							60≤l _ü ≤l _{ü,ma}																
	[]				h _{min}	l _{ü,max}	l _{ü,max}	h _{min}	h _{min}	h _{min}																	
HBT 80 3		10		170		170	170				70																
A	8	15 20	170	320	120	360 450	360 450	120	135	140	125 175																
A D				320																							
		25		100		500	480				225																
~		10 15		160		160 320	160 320				60 120																
	10	20	170	320	120	400	400	120	155	140	120																
		20		390		400	400				220																
1107 400		10		290		290	290				115																
HBT 120		15		290		510	430				175																
	8	20	170	320	120	510	455	120	135	140	235																
Contraction of the second		25		520		600	480				290																
	/	10		240		240	240				80																
V		15		240		510	425				130																
	10	20	170	390	120		420	120	155	140	180																
		20		550		600	475				230																
		10		215		215	215				70																
		15		15		390		390	390				120														
	12	20	170	440	120	440	440	120	170	140	120																
												20		460		490	470				220						
HBT 150		10		400		360	360						100														
		15				510	430				150																
	8	20								170	320	120		455	120	135	140	210									
C. and C.		25				600	480				260																
le la		10		360		360	360				85																
		15		500		510	425				135																
	10	20	170	390	120		450	120	155	140	185																
		25		050		600	475				235																
		10		310		310	310				90																
		15			480	420	420			150																	
	12	12	12 20	12	12 20	20		12 20	12	12	12	12	12	10	12	12	12		170	460	120	530	445	120	170	140	200
		25				580	470				250																
HBT 190		10				500	405				100																
1101 150		15				510	430				150																
	8	20	170	320	120	600	455	120	135	140	210																
		25				600	480				260																
		10				500	400				110																
i ser	10	15	470	200	120	510	425	120	455	140	170																
	10	20	170	390	120	600	450	120	155	140	220																
		25				600	475				280																
		10		430		430	395				90																
	10	15	170		120	510	420	120	170	140	140																
	12	20	170	460	120	600	445	120	170	140	190																
		25				600	470				240																
HBT 220		10					405				100																
	8	15	170	320	120	600	430	120	135	140	150																
	0	20	170	520	120	000	455	120	661	140	210																
		25					480				260																
Contraction of the second seco		10					400				110																
	10	15	170	390	120	600	425	120	155	140	170																
V	10	20	170	590	120	000	450	120	201	140	220																
		25					475				280																
		10					395				90																
	12	15	170	460	120	600	420	120	170	140	140																
	12	12 20 170	400	120	000	445	120	170	140	190																	
		25					470				240																

() rebar type 7 for HBT 80 not available () Due to the required bending roll diameter (d_{Br} = 10x bar diam.); h_{min} for type 23 is equal to h_{min} of type 7

Product Selection/Calculation Basis according to Approval no. Z-21.8-2035

Product selection all	roduct selection allowing for required concrete cover of the rebend reinforcement											
	Thickness of component D ₂ [mm]											
profile	100	120	140	160	180	200	220	240	260	280	300	
		concrete cover c _{nom} [mm]										
2×HBT 55 ①	-	-	≤ 25	≤ 35	≤ 45	≤ 55	≤ 65	≤ 75	≤ 85	≤ 95	≤ 105	
2×HBT 85 ①	-	-	-	-	≤ 20	≤ 30	≤ 40	≤ 50	≤ 60	≤ 70	≤ 80	
HBT 80	20	30	40	50	60	70	80	90	100	110	120	
HBT 120	-	-	25	35	45	55	65	75	85	95	105	
HBT 150	-	-	-	20	30	40	50	60	70	80	90	
HBT 190	-	-	-	-	-	23	33	43	53	63	73	
HBT 220	-	-	-	-	-	-	-	25	35	45	55	

depends on spacing a between the cases



Product selection for 1-part element



Product selection for multi-part elements

Basis for calculation according to approval number Z-21.8-2035

General information

The concept for the approval is based on the calculation and the structural application as applied in the following standards and guidelines: DIN EN 1992-1-1 with DIN EN 1992-1-1/NA (National Annex) and the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2". Generally two different cases of shear load are examined: shear load transverse and longitudinal to the concrete joint.

According to the approval, due to the product characteristics the shear loads transverse to the joint in the HBT Rebend connections can be classed as "indented" and shear loads longitudinal to the joint can be classified as "rough".

Material: Rebend reinforcement B500B, stainless steel B500 NR with bar diameters of 8, 10 and 12 mm Normal concrete \geq C20/25

The maximum load bearing capacity of the rebend connection must be limited to 80% of the ultimate limit state; the following applies for tensile strength:

$$f_{yd,red} = 0.8 \cdot \frac{f_{yk}}{\gamma_s}$$

Existing anchorage lengths and overlap lengths must be taken into consideration for the calculation. These can be calculated from the back surface of the case. Verification of the anchorage lengths and overlap lengths is according to Eurocode 2, taking the bonding characteristics into account.



Anchorage and overlap length requirements for the reinforcement

Calculating the HALFEN HBT Rebend connections as in the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2" with roughness classification "smooth", is conservative while still being acceptable.

Calculation Basis according to Approval no. Z-21.8-2035

Shear load, transverse to the concrete joint

Calculation is according to DIN EN 1992-1-1, section 6.2 and DIN EN 1992-1-1/NA, as for monolithic produced building components; whereby the following additional provisions must be observed.

Effective static height:





Tension zones: upper and lower component edges

Shear resistance with no shear reinforcement

The decisive resistance $V_{Rd,c}$ for the calculation is according to DIN EN 1992-1-1, section 6.2.2; whereby a reduction in the reinforcement ratio ρ_l is not required (caused by the reduced yield strength of the rebend reinforcement).



Shear load, longitudinal to the concrete joint

Static verification is according to DIN EN 1992-1-1, section 6.2.5 and DIN EN 1992-1-1/NA (National Annex), whereby the following additional provisions are to be observed. Factors to used to calculate c = 0.4the shear load resistance: $\mu = 0.7$







Combined shear load, transverse and longitudinal to the concrete joint

If the connection is subjected to combined shear load (longitudinal and transverse to the concrete joint), each load direction can be verified separately.

Shear resistance with shear reinforcement

The decisive resistance for verification results from 30% of the shear load resistance $V_{Rd,max}$ according to DIN EN 1992-1-1, section 6.2.3:



$V_{Ed} \le V_{Rd} \le 0.3 \cdot V_{Rd,max}$

An additional load in the longitudinal reinforcement caused by shear forces must be verified assuming a compression strut angle of 45° (cot θ = 1.0). The required shear reinforcement is calculated using cot θ = 1.0

To calculate the shear load reinforcement for HALFEN HDB-S Shear rails see HDB technical product information and the approvals Z-15.1-249 and Z-15.1-270.

If the spaces between the HBT Cases are additionally subjected to shear load, then these must be designed as rough or suitably indented in accordance with DIN EN 1992-1-1. The edge areas can also be assumed as load bearing if $a_1 \ge 50$ mm.

The concrete cover c_{nom} is according to DIN to EN 1992-1-1. In addition for the rebend reinforcement the following must be observed.

 $c_{nom} \ge max. \{3\emptyset, 30 \text{ mm}, max. aggregate diameter } d_g\}$

TECHNICAL CONSULTATION

Technical services

See inner back of catalogue for technical advice for your individual projects and contact information for all products.

Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

Shear load capacity transverse to the case — no shear reinforcement

Single connection example; 1 × Type 5



Multipart connection example; 2 × Type 1



ear load resist	ance V _{Rd} [kN	I/m] (≤ 0.3 · V _I	Rd,max)				Alwa	ys refer to th	e information i	n the type tes	
rebarØ/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mn	
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55 (Ø8) 2×85 (Ø10,1	
				concrete	strength cla	ss C20/25					
8/20	44.3	53.1	62.0	70.8	78.7	78.7	78.7	78.7	78.7	78.7	
8/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	104.9	
8/10	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8	
10/20	44.3	53.1	62.0	70.8	79.7	88.5	94.0	98.4	98.4	98.4	
10/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8	
10/10	-	56.6	62.7	70.8	79.7	88.5	94.0	99.4	104.7	109.8	
12/20	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8	
12/15	-	55.8	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8	
12/10	-	63.9	70.8	77.4	83.7	89.8	94.0	99.4	104.7	109.8	
concrete strength class C25/30											
8/20	49.5	59.4	69.3	78.7	78.7	78.7	78.7	78.7	78.7	78.7	
8/15	49.5	59.4	69.3	79.2	89.1	99.0	104.9	104.9	104.9	104.9	
8/10	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8	
10/20	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	114.2	114.2	
10/15	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8	
10/10	-	60.9	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8	
12/20	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8	
12/15	-	60.1	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8	
12/10	-	68.8	76.3	83.4	90.2	99.0	105.1	111.1	117.0	122.8	
				concrete	strength cla	ss C30/37					
8/20	54.2	65.1	75.9	78.7	78.7	78.7	78.7	78.7	78.7	78.7	
8/15	54.2	65.1	75.9	86.8	97.6	104.9	104.9	104.9	104.9	104.9	
8/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5	
10/20	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	122.9	122.9	
10/15	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5	
10/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5	
12/20	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5	
12/15	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5	
12/10	-	73.1	81.0	88.6	97.6	108.4	115.1	121.7	128.2	134.5	

Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

Shear load capacity transverse to the case — with shear reinforcement

Single connection example; $1 \times Type 5$



h V_{Ed}

Multipart connection example; 2 × Type 1

Please refer to the HDB Product information and approvals no. Z-15.1-249 and Z-15.1-270 when determining the shear reinforcement using HALFEN HDB-S Shear reinforcement.

Shear load re	sistance V _{Rd} [k	:N/m] (≤ 0.3 · \	V _{Rd,max})				Alwa	lys refer to th	e information ir	the type test		
rebar Ø/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mm		
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55(Ø8) 2×85(Ø10,12)		
				concrete	e strength cla	iss C20/25						
8/20	76.5	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4		
8/15	76.5	102.0	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6		
8/10	-	102.0	127.5	153.0	174.8	174.8	174.8	174.8	136.5	136.5		
10/20	76.5	102.0	109.4	109.4	109.4	109.4	109.4	109.4	109.4	109.4		
10/15	76.5	102.0	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8		
10/10	-	102.0	127.5	153.0	178.5	204.0	218.7	218.7	218.7	218.7		
12/20	-	102.0	125.6	125.6	125.6	125.6	125.6	125.6	125.6	125.6		
12/15	-	102.0	127.5	153.0	167.5	167.5	167.5	167.5	167.5	167.5		
12/10	-	102.0	127.5	153.0	178.5	204.0	229.5	251.2	251.2	251.2		
	concrete strength class C25/30											
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4		
8/15	95.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6		
8/10	-	127.5	159.4	174.8	174.8	174.8	174.8	174.8	158.4	158.4		
10/20	95.6	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9		
10/15	95.6	127.5	159.4	169.2	169.2	169.2	169.2	169.2	169.2	169.2		
10/10	-	127.5	159.4	191.3	223.1	253.8	253.8	253.8	253.8	253.8		
12/20	-	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8	145.8		
12/15	-	127.5	159.4	191.3	194.4	194.4	194.4	194.4	194.4	194.4		
12/10	-	127.5	159.4	191.3	223.1	255.0	286.9	291.5	291.5	291.5		
				concrete	strength cla	iss C30/37						
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4		
8/15	114.8	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6		
8/10	-	153.0	174.8	174.8	174.8	174.8	174.8	174.8	174.8	174.8		
10/20	114.8	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6		
10/15	114.8	153.0	182.1	182.1	182.1	182.1	182.1	182.1	182.1	182.1		
10/10	-	153.0	191.3	229.5	267.8	273.2	273.2	273.2	273.2	273.2		
12/20	-	153.0	164.6	164.6	164.6	164.6	164.6	164.6	164.6	164.6		
12/15	-	153.0	191.3	219.5	219.5	219.5	219.5	219.5	219.5	219.5		
12/10	-	153.0	191.3	229.5	267.8	306.0	329.2	329.2	329.2	329.2		
	d dimonsion as		7 10									

Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

Basis of Calculation According to Type Test and Approval no. Z-21.8-2035

Shear load capacity, longitudinal to the case - single row profile

Multipart connection example; 2 × Type 1 Standard type according to page 7





Shear load resistance V _{Rdi} [kN/m] (≤ V _{Rdi,max}) Always refer to the information in the type test												
profile	b=260 mm			2×HBT 55 (Ø8) 2×HBT 85 (Ø10,12) b=300mm b=360m			Ø 10, 12) b=360 mm	n b=400 mm				
rebarØ[mm]/					c	oncrete st	ength clas	S				
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37
8/20	193.4	224.4	253.4	207.4	240.7	271.8	228.5	265.1	299.4	242.5	281.4	317.7
8/15	227.4	263.9	298.0	241.5	280.2	316.4	262.5	304.6	344.0	276.6	320.9	362.4
8/10	295.6	343.0	387.3	309.6	359.3	405.7	330.7	383.7	433.3	344.7	400.0	451.7
10/20	213.7	248.0	280.0	227.7	264.2	298.4	248.8	288.7	326.0	262.8	304.9	344.3
10/15	254.5	295.3	333.5	268.5	311.6	351.9	289.6	336.0	379.5	303.6	352.3	397.8
10/10	336.2	390.1	440.5	350.2	406.4	458.9	371.3	430.8	486.5	385.3	447.1	504.9
12/20	231.9	269.1	303.9	245.9	285.4	322.2	267.0	309.8	349.8	281.0	326.1	368.2
12/15	278.8	323.5	365.3	292.8	339.8	383.7	313.9	364.2	411.3	327.9	380.5	429.6
12/10	372.6	432.3	488.2	386.6	448.6	506.6	407.7	473.0	534.2	421.7	489.3	552.6

Note: Load capacities for further joint widths, for further rebar dimensions and for rebar spacings of 25 cm, see type test. The joint areas a between the HBT Cases must be designed as rough or suitably indented as defined in DIN EN 1992-1-1; 6.2.5.

Shear load capacity, longitudinal to the case – double-row profile

Single connection example; 1 × Type 5 Standard type according to page 9





Shear load resistance V _{Rdi} [kN/m] (≤ V _{Rdi,max}) for HBT 120, 150, 190 and 220 Always refer to the inf									o the inforr	nation in th	e type test	
profile	HBT 120			HBT 150			HBT 190			HBT 220		
rebarØ[mm]/						oncrete st	-					
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37
8/20	145.0	168.2	190.0	154.8	179.6	202.9	167.4	194.3	219.4	180.1	208.9	235.9
8/15	179.1	207.8	234.6	188.9	219.2	247.5	201.5	233.8	264.0	214.1	248.5	280.6
8/10	247.2	286.8	323.9	257.0	298.2	336.8	269.6	312.9	353.3	282.3	327.5	369.9
10/20	165.3	191.8	216.6	175.1	203.2	229.5	187.7	217.8	246.0	200.4	232.5	262.5
10/15	206.1	239.2	270.1	215.9	250.6	283.0	228.6	265.2	299.5	241.2	279.9	316.0
10/10	279.0	323.8	365.6	297.6	345.3	390.0	310.2	360.0	406.5	322.9	374.6	423.1
12/20	183.5	212.9	240.4	193.3	224.3	253.3	205.9	239.0	269.8	218.6	253.6	286.4
12/15	230.4	267.3	301.9	240.2	278.7	314.8	252.8	293.4	331.3	265.5	308.0	347.8
12/10	306.3	355.5	401.4	334.0	387.6	437.7	346.6	402.2	454.2	359.3	416.9	470.8

Note: Load capacities for the HBT 80 profile; for further rebar dimensions and for rebar spacings of 25 cm, see type test

Calculating Reinforced Concrete Corbel

Calculating a reinforced concrete corbel for HBT Type 7



Strutural boundary conditions

Geometric assumptions:
$$0.2 \le \frac{a_c}{h_c} \le 1.0$$

Anchorage length in the corbel:

$$I_{bd,dir} = \frac{2}{3} I_{b,eq} \ge \max \begin{cases} 0.67 \cdot \alpha_1 \cdot \alpha_4 \cdot I_{b,rqd} \\ 6.7 \emptyset \end{cases}$$

Shear load capacity in the corbel:

$$F_{Ed} \le V_{Rd,max} = 0.5 \cdot v \cdot b_c \cdot z \cdot \frac{r_{ck}}{\gamma_c}$$

where $v = 0.7 - \frac{f_{ck}}{200 \text{ N/mm}^2} \ge 0.5$
 $z = 0.9 \cdot d$

Tensile load in the corbel:

$$\begin{aligned} Z_{Ed} &= F_{Ed} \cdot \frac{a_{C}}{z_{o}} + H_{Ed} \cdot \frac{a_{H} + z_{o}}{z_{o}} \\ \text{where } z_{o} &= d \cdot \left(1 - 0.4 \cdot \frac{F_{Ed}}{V_{Rd,max}}\right); \ H_{Ed} \geq 0.2 \cdot F_{Ed} \\ \text{and } \frac{a_{C}}{z_{o}} &\geq 0.4 \end{aligned}$$

Required reinforcement for tension

$$a_{s,rqd} = min \begin{cases} \frac{Z_{Ed}}{0.8 \cdot f_{yd}} \\ \frac{F_{bd}}{0.8 \cdot f_{yd}} \end{cases}; where F_{bd} = anchored load$$

Verifying the reinforcement overlap length in the wall for:

$$I_{0,avail.} = b_1 - 6\emptyset$$

In addition to the corbel calculation above, the rebend connection has to be verified according to the specifications in approval no. Z-21.8-2035 (see page 10 f).

The compressive stresses under the load application plate and the wall's joint load bearing capacity must be verified in accordance with DIN EN 1992-1-1 resp. publication no. 532 issued by the German Committee for Structural Concrete (DAfStb).

The spacing between the side-edge of the corbel and the outermost stirrups in the HBT Connection should not exceed 5 cm. The free side-edge of the corbel must be strengthened using stirrups.



Concrete strength class C30/37 c _{nom} =35 mm										
		h _c =								
	reinforce-	160 mm	190 mm	230 mm	250 mm					
	ment	max F _{Ed} for profile type [kN/m]								
	Ø/s	HBT 120	HBT 150	HBT 190	HBT 220					
$L_{c} = 200 \text{ mm}$	8/25	51.7	61.7	61.4	61.3					
l _ü = 165 mm	8/20	64.2	76.9	77.1	77.0					
a _c = 100 mm	8/15	64.5	-	-	-					
	10/25	63.7	76.4	91.8	96.7					
	10/20	64.0	80.1	101.5	110.1					
	10/15	-	-	101.5	110.1					
L _c = 220 mm	8/25	53.4	61.5	61.2	61.1					
l _ü = 185 mm	8/20	64.4	77.3	77.0	76.8					
a _c = 110 mm	10/25	63.9	79.1	95.4	96.5					
	10/20	-	79.9	101.3	110.0					
	12/25	63.3	79.4	100.8	109.6					
	12/20	-	79.4	100.8	109.6					
L _c = 240 mm	8/25	53.3	61.4	61.1	60.9					
l _ü =205 mm	8/20	64.3	77.1	76.8	76.7					
a _c = 120 mm	10/25	63.8	79.8	96.5	96.3					
	10/20	-	-	101.2	109.8					
	12/25	63.2	79.3	100.6	109.5					
L _c = 260 mm	8/25	50.1	60.5	60.9	60.7					
l _ü =225 mm	8/20	62.2	-	-	-					
a _c = 130 mm	10/25	63.7	79.7	96.3	96.2					
	12/25	-	79.1	100.5	109.3					
L _c = 280 mm	8/25	47.3	57.2	60.8	60.6					
l _ü =245 mm	10/25	-	-	96.2	96.0					
a _c = 140 mm	12/25	-	79.0	-	-					

 $H_{Ed} = 0,2 \cdot F_{Ed}$; h = 170 mm, b₁=400 mm load-plate a_L = 50 mm, d_L = 10 mm

These values are for pre-dimensioning; final values must be verified. Further dimensions and resistance values are available on request. Generally, significantly better resistance values are possible when using the HALFEN HSC Stud connector.



Layout of the Reinforcement in the Case



Minimal element lengths for the rebend connections – layout of the reinforcement bars in the case

Minimal e	Minimal element lengths for l _ü standard								
Ø [mm]	rebar spacing s [cm]	① l _ü [mm]	minimal case length L [mm]	number of rebar					
8	10	320	650	6					
8	15	320	600	4					
8	20	320	650	4					
10	10	390	800	8					
10	15	390	750	4					
10	20	390	700	4					
12	10	460	950	8					
12	15	460	900	6					
12	20	460	850	4					
1 Max. I _ü -	observe dimens	ions for select	ted profile (see pag	e 7 and 9)					

HBT Strip — the perfect solution for precast elements

Due to inherent low concrete cover and restriction in element size in precast elements, demands on precise measurement and manufacturing of rebend connections are generally higher.

The HBT-Connection with 'HBT Strip' (HBT-S) fulfils these requirements perfectly. Available for all double-row HBT connections with rebar types 6, 7, 8 and 9, in profile widths HBT - S 80, 120, 150, 190, 220.

- rebars are secured during transport
- · compliance with required over-lap and anchorage lengths
- recommended for loops and corbel connections
- HBT Strip is easily removed after striking the formwork

Order example:



Installation is as described on page 18; the HBT Strip must be removed after all connecting rebar have been rebent.



Application Suggestions

Application of short elements

Various lengths - efficient installation times

Combining 1250 mm standard elements and 800 mm elements helps to avoid unnecessary modification of HBT Elements.

Standard combinations								
	clear floor height [m]							
HBT Element	2.40	2.50	2.85	3.30	3.75			
	combinations							
short element L = 800 mm	3	-	2	1	-			
standard element L = 1250 mm	-	2	1	2	3			

In numerous applications, further on-site modification of the HBT cases is therefore not required. The risk of damaging the rebar in the casing is avoided. The planner can plan more efficiently and on-site preparation time for installation of the HBT Elements is reduced.



Wall connection using two 1250 mm HBT Elements and a 800 mm HBT Element to obtain 3.30 m floor height

In combination with HBS-05

Combined strength!

In applications with concentrated shear loads HBT Rebend connections in combination with the proven HALFEN HBS-05 Screw connections is feasible. HBS-05 Connection rebars can be used if the calculated shear load in an area exceeds the allowable characteristic resistance values for the HBT Profiles. The surface characteristics in the area of the HBS-05 Screw connections must be designed as "rough" or "indented". Verification of the shear load transfer is according to DIN EN 1992-1-1, 6.2.5. The result is a **cost effective** solution and **efficient** installation.

Assembly Instructions

Assembly instructions









- 1. Nail the HBT Element to the timber formwork in the specified position. Use suitable methods for fixing elements to metal formwork, for example magnets. Check stirrup lengths protruding from the case after installation.
- 2. After the concrete has cured, strike the formwork; hitting a wood block placed in the long groove in the cover with a hammer, loosen the cover.
- **3.** Use a hammer with a claw to punch-in the perforated hole in the cover; hook the hammer in the hole and pull the lid out.
- 4. Place the Rebending tool under the rebar and pull down on the handle evenly with both hands until the rebar is in the correct position. The bar must be rebent straight without any kinks. Proceed in a similar manner to rebend all bars in the HBT Element one-by-one. The profiled back of the HBT Element case remains in the concrete.

Note: A more detailed installation instruction can be found at www.halfen.com.

HBT Element adapted to curved formwork



Adapting the HBT Case

Using an angle-grinder cut approximately 1 cm deep incisions symmetrically into both sides of the case at regular intervals; the HBT housing loses its rigidity, easing fixing to the formwork. To achieve a better fit to smaller curvature (< 3.00 m), up to seven incisions per side are possible.

After fixing the HBT Case to the formwork cover the incisions with adhesive tape to prevent concrete seeping into the form.

Caution when working with an angle-grinder! The reinforcing steel bars in the HBT Case must not be damaged.



HBT Element fitted to an convex curvature Outer radius \geq ca. 3.00 m; smaller radius is achieved with more incisions.



HBT Element fitted to an concave curvature Inner radius \geq ca. 3.00 m; smaller radius is achieved with more incisions.

Order Examples

Order examples

We offer a wide range of standard HBT Rebend connections for the most common applications (see page 6ff.). There is also a wide product range with corresponding profile widths and rebar shapes to choose from. Rebar dimensions and element lengths are freely definable, limited only by geometric specifications and limits in production (see also page 16).



HBT Connection with foam strip filler "HBT Strip" (see page 16).

Order form

Туре 6



Pos.	profile HBT	rebarØ [mm]	rebar spacing s [cm]	stirrup type	element length L[mm]	h [mm]	l _ü [mm]	1×v y	value if pre b	in [mm esent b₁ b	d _{Br}	total length [m]	number of elements [items]

Туре 8

Note: do not exceed $I_{\ddot{u},max}$ see table on page 7 and 9; see page 6 to 9 for rebar and profile dimensions.

Туре 7



Company	Fax or email
Address	this form
City, Postcode/Zip.	to us.
Tel. no.	See inner
E-Mail	back-cover
Fax	for addresses.



Туре 9

Tender Texts

HALFEN Rebend connection, type HBT 85-10/20-1-1250

HALFEN HBT Rebend connection with single-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

Type HBT 85 - 10/20 - 1 - 1250with 85 = type identifier for a case width of 85 mm with a single-row of rebar, 10/20 = reinforcement steel B500B with 10 mm bar diameter and 200 mm bar spacing, 1 = standard rebar type 1, 1250 = element length [mm], in standard rebar dimensions rebar length 1 h = 170 mm, rebar length 2 $l_{\ddot{u}}$ = 390 mm, bend length y = 95 mm,



or equivalent; deliver and install according to the manufacturer's instructions.

HALFEN Rebend connection, type HBT 150-12/15-5-1250

HALFEN HBT Rebend connection with double-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

Type HBT 150 - 12/15 - 5 - 1250with 150 = type identifier for a case width of 150 mm with a double-row of rebar, 12/15 = reinforcement steel B500B with 12 mm bar diameter and 150 mm bar spacing, 5 = standard rebar type 5, 1250 = element length [mm], in standard rebar dimensions rebar length 1 h = 170 mm, rebar length 2 l_{ii} = 460 mm,



or equivalent; deliver and install according to the manufacturer's instructions.

HALFEN Rebend connection, with stainless steel reinforcement bars

Use the text as above, but replace "B500B" with "stainless steel B500 NR according to building authority approval".

Further tender texts can be found at www.halfen.com.

Further HALFEN Reinforcement Products

HALFEN HBS-05 Screw connections

The HALFEN HBS-05 Screw connections allow rebar continuity joints possible with simple screw and socket rebar. Their versatility allows nearly every type of reinforcement joint. HALFEN HBS-05 fulfils German and international certification criterion. Extensive certification and test reports prove their suitability also under extreme conditions.





No torque wrench or special tools required – visual check is sufficient

HALFEN HSC Stud Connector

The HALFEN HSC Stud connector is a building authority approved reinforcement optimized for anchorage in concrete. Maximum exploitation of the reinforcement is possible with extremely short anchoring lengths.

HALFEN HSC Anchor is especially advantageous for use in high-density reinforced areas, for example; corbels and frame corner nodes.





HALFEN HLB Loop Box

The HALFEN HLB Loop Box is an efficient and time saving method for connecting concrete elements. HLB Loop boxes are cast into concrete elements (e.g. wall elements) in the precast plant. The prepared concrete elements are transported to site, lifted and correctly placed with a crane; the joint between the elements is then cement-grouted.





References



Kö-Bogen, Düsseldorf, Germany

The Kö-Bogen complex designed by Daniel Libeskind offers commercial space for business as well as a shopping centre with restaurants etc. The Kö-Bogen (curve) is located at the end of the Königsallee in Düsseldorf, Germany.

We supplied HALFEN HBT Rebend connections for the Kö-Bogen. Other HALFEN products also used in this project include;

- HALFEN HTA Channels
- HALFEN Masonry connection channels
- HALFEN HDB Shear rails
- HALFEN HBS-05 Screw connections



Kopernikus Science centre, Warsaw, Poland

The Science centre in Warsaw, Poland, named after Nikolaus Kopernikus, is a collection of buildings with six exhibition areas.

We supplied HALFEN HBT Rebend connections for this project.



DATEV IT-Campus, Nuremberg, Germany

The DATEV IT-Campus on Fürther Strasse in Nuremberg offers 1800 workspaces including 200 individual offices and conference rooms of various sizes. The building was inaugurated in 2015.

We supplied HDB Shear rails and HBT Rebend connections for this project.



11



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